		TOTAL	NEW CIP
	2023	CIP + MAINT BORROW	BORROW ONLY
VILLAGE LEVY	1,610,916.30	1,793,528	1,671,606
VILLAGE MILLRATE	0.005029324	0.005599442	0.005218799
NET MILLRATE	0.014400791	0.01497091	0.014590266
HOME VALUE			
100,000.00	1,440.08	57.01	18.95
200,000.00	2,880.16	114.02	37.90
250,000.00	3,600.20	142.53	47.37
300,000.00	4,320.24	171.04	56.84

Structural Summary Information and Design Loads

Project Number:	24A05	Project Name:	Osceola Fair Grounds
Municipality:	Village of Osceola, County: Polk, WI		

Design Focus		Project-specific comments	Project Values
General Information:	Risk Category =	*Normal Occupied Building*	II
	Concrete Compressive Strength (f_c') =	*28 day minimum strength*	3500 psi
Roof Dead Loads:	TC_{Dead}	*Top Chord Dead Load*	4.0 *psf*
	BC_{Dead}	*Bottom Chord Dead Load*	5.0 *psf*
§1603.1.3 Roof Snow Load Data:			
	Ground Snow Load (P_g) =	*Meets or exceeds ASCE 7 value*	60.0 psf
	Snow Exposure Factor (C_e) =		1.0
	Snow Load Importance Factor (I_s) =	*Based on Risk Category above*	1.0
	Thermal Factor (C_t) =		1.2
	Flat Roof Snow Load (P_f) =	$0.7 \times C_e \times I_s \times C_t \times P_g$	50.4 psf
	Sloped Roof Factor (C_s) =	*Slippery assumption may be invalid*	1.00
	Balanced Roof Snow Load (P_s) =	$C_s \times P_f$ (or greater)	50.4 psf
	Unbalanced Roof Snow Load =	(Windward / Leeward)	Not Required
		See Detailed Snow Load Sheet for more details	
§1603.1.4 Wind Design Data:			
	Basic Wind Speed =	*ASCE 7-16 speed is Strength / Ultimate*	115 mph
	Wind Exposure =	*Presumed to apply in All Directions*	C
	Building Enclosure Type =		Enclosed
	Velocity Pressure at nominal roof height "h", q_h:		**25.3 psf**
	Internal Pressure based on Enclosure Type (q_h *GC_{pi}*):		±4.4
	Maximum C&C Pressure in Roof Zones 1 / 2 / 3 ($q_h GC_p$):		-22.1 / -41.7 / -63.7 psf
	Maximum C&C Pressure in Wall Zones 4 / 5 ($q_h GC_p$):		-27 / -34.3 psf
§1603.1.5 Earthquake Design Data:			
	Seismic Importance Factor I_e =	*Based on Risk Category above*	1.0
	Mapped spectral response parameters:	*Max. Value for Zip Code*	S_s = 5.5%
			S_1 = 2.5%
	Site Class =	*Presumed in absence of soils test*	D
	Design Spectral Coefficients:	$S_{DS} = (2/3) \times F_a \times S_s$	S_{Ds} = 5.9%
		$S_{D1} = (2/3) \times F_v \times S_1$	S_{d1} = 4.0%
	Seismic Design Category =		A
	Basic Seismic Force-Resisting System =		A15 - Light-frame walls with shear panels (other)
	Seismic Response Coefficient =		C_s = 0.029
	Response Modification Coefficient =	*(6.5 might be OK for wood frame)*	R = 2
	Effective Seismic Weight =	Dead Load x Area	W = 67,392 lbs
	Design Base Shear =	$V = C_s \times W$	V_s = 1,977 lbs
	Analysis Procedure Used =		Equivalent Lateral Force Procedure
§1603.1.6 Geotechnical Information:			
	Class of Soil Materials =	*Presumed in absence of soils test*	4 Firm
	Allowable Soil Pressure =	*As Defined in EP486.1*	2000 psf
	Shallow Post Foundation Design (ANSI/ASAE EP486.1 OCT00), adopted in IBC 2018 §2306.1		
§1603.1.7 Flood Hazard Information:		*Not Considered*	
§1603.1.8 Special Loads:		*Not Applicable*	
§1603.1.9 Special Seismic Inspections		*None Required*	

Building Design Snow Loads

Balanced Snow Load Design: **ASCE 7-16**
Unbalanced Snow Load Design: Wisconsin Alternate

Project Information

Project Number:	24A05
Project Name / Description:	Osceola Fair Grounds - 52'x144' Storage with office and 16'x1
Location	Village of Osceola, County of Polk, WI

Roof Snow Loads to be used in Load Combinations for Truss and Building Design:

I	Balanced Snow Load (P_s)	Over entire roof at once	50.4	psf

Is the Unbalanced Snow Load Analysis required for this project?			No	
II	Unbalanced Snow Load Analysis Not Required	Windward Roof	Not	psf
		Leeward Roof	Req'd	psf

Project Information to determine snow loads listed above:

Roof Pitch (inches per foot)	3	/12
Roof Surface	Other	(see ASCE 7, §7.4)
Ground Snow Load, P_g	60	psf
Snow Exposure Factor, C_e	1.0	(see ASCE 7, Table 7-2)
Thermal Condition Factor, C_t	1.2	(see ASCE 7, Table 7-3)
Snow Load Importance Factor, I_s	1.0	(see ASCE 7, Table 7-4)
Flat Roof Snow, $P_f = 0.7\ C_e C_t I_s P_g$	50.4	psf
Roof Slope Factor, C_s	1.000	(see ASCE 7, §7.4)
Half Roof Width (Ridge to Eave), W:	28.0	ft
Building Length, L:	148.0	ft
Snow Density, g	21.8	pcf

Snow Drift Analysis Required at High / Low condition (per ASCE 7-16, Section 7.7.1) ?	No
Sliding Snow Analysis required from Upper to Lower Roof (per ASCE 7-16, Section 7.9) ?	No

Building Design Wind Loads

Based upon ASCE 7-16, Chapters 26, 28, and 30 - Envelope Procedure for Low-rise buildings, Part 1 - Specified Equations

Least Horizontal Dimension:	52 ft	Project #:	24A05
Eave Height:	14.33 ft	Name:	Osceola Fair Grounds
Edge Strip Width (a):	5.20 ft	Description (opt.):	52'x144' Storage with office and 16
End Zone Width (2a):	10.40 ft		

Velocity Pressure (Low Profile uses q_h throughout) - Low-rise, rigid building assumptions

Exposure (B/C) =	C	Basic Wind Speed, V =	115 mph
K_e =	0.88	Mean roof height (h) =	17.58 ft
q_h =	24.5 psf	Sidewall Column Spacing =	6 ft
Building Slope =	3 /12	Building Site Elevation ASL =	830 ft
		K_d	0.97

Pressures reported at STRENGTH level (Load combination will reduce by 0.6 factor for ASD calculations)

MWFRS (ASCE 7, Chapter 28, Part 1)

Load Case A - External Pressure Coefficients by Zone (refer to ASCE 7-16, Figure 28.4-1)

	1	2	3	4	1E	2E	3E	4E
GC_{pf}	0.48	-0.69	-0.44	-0.37	0.72	-1.07	-0.63	-0.56
$q_h \cdot GC_{pf}$ (psf)	11.7	-16.9	-10.7	-9.2	17.8	-26.2	-15.4	-13.6
Minimum Pressure (§28.4.4)	16.0	8.0	0.0	0.0	16.0	8.0	0.0	0.0

Load Case B - External Pressure Coefficients by Zone (refer to ASCE 7-16, Figure 28.4-1)

	1	2	3	4	5	6	1E	2E	3E	4E	5E	6E
GC_{pf}	-0.45	-0.69	-0.37	-0.45	0.40	-0.29	-0.48	-1.07	-0.53	-0.48	0.61	-0.43
$q_h \cdot GC_{pf}$ (psf)	-11.0	-16.9	-9.1	-11.0	9.8	-7.1	-11.8	-26.2	-13.0	-11.8	15.0	-10.5
Minimum Pressure (§28.4.4)	0.0	0.0	0.0	0.0	16.0	0.0	0.0	0.0	0.0	0.0	16.0	0.0

Internal Pressure Coefficient: (§28.4.1. and §26.11.1)

Building Enclosure	Enclosed
GC_{pi} (±)	0.18
$q_h GC_{pi}$ (±)	**4.4** psf

Pressure on Windward Eave O-Hang (§28.4.3)

C_p	0.7
G (assumed)	0.85
Pressure: $q_h C_p G$	**14.6** psf

Components and Cladding (ASCE 7, Chapter 30, Part 1), values based on zone and eff. area (Fig. 30.4-1 & 30.4-2B)

GC_p Net EXTERNAL coefficients, *Pressures in psf at STRENGTH levels* :

Roof Elements (for	≤ 10sf		btw	≥ 100sf	
Zone 1	0.50	*12.3*	interpolate	0.30	*7.4*
	(0.90)	*(22.1)*		(0.80)	*(19.6)*
Zone 2	0.50	*12.3*		0.30	*7.4*
	(1.70)	*(41.7)*		(1.20)	*(29.4)*
Zone 3	0.50	*12.3*		0.30	*7.4*
	(2.60)	*(63.7)*		(2.00)	*(49.0)*
Roof Overhang C&C	≤ 10sf		btw	≥ 100sf	
Zone 2	(2.2)	*(53.9)*	<same>	(2.2)	*(53.9)*
Zone 3	(3.7)	*(90.7)*	<interp>	(2.5)	*(61.3)*

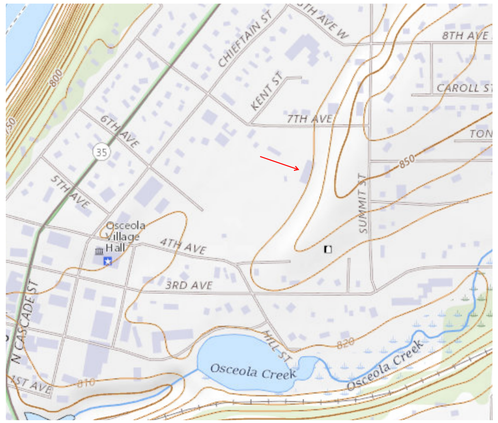
Wall Elements	≤ 10sf		btw	≥ 500sf	
Zone 4	1.00	24.5	interpolate	0.70	*17.2*
	(1.10)	*(27.0)*		(0.80)	*(19.6)*
Zone 5	1.00	*24.5*		0.70	*17.2*
	(1.40)	*(34.3)*		(0.80)	*(19.6)*

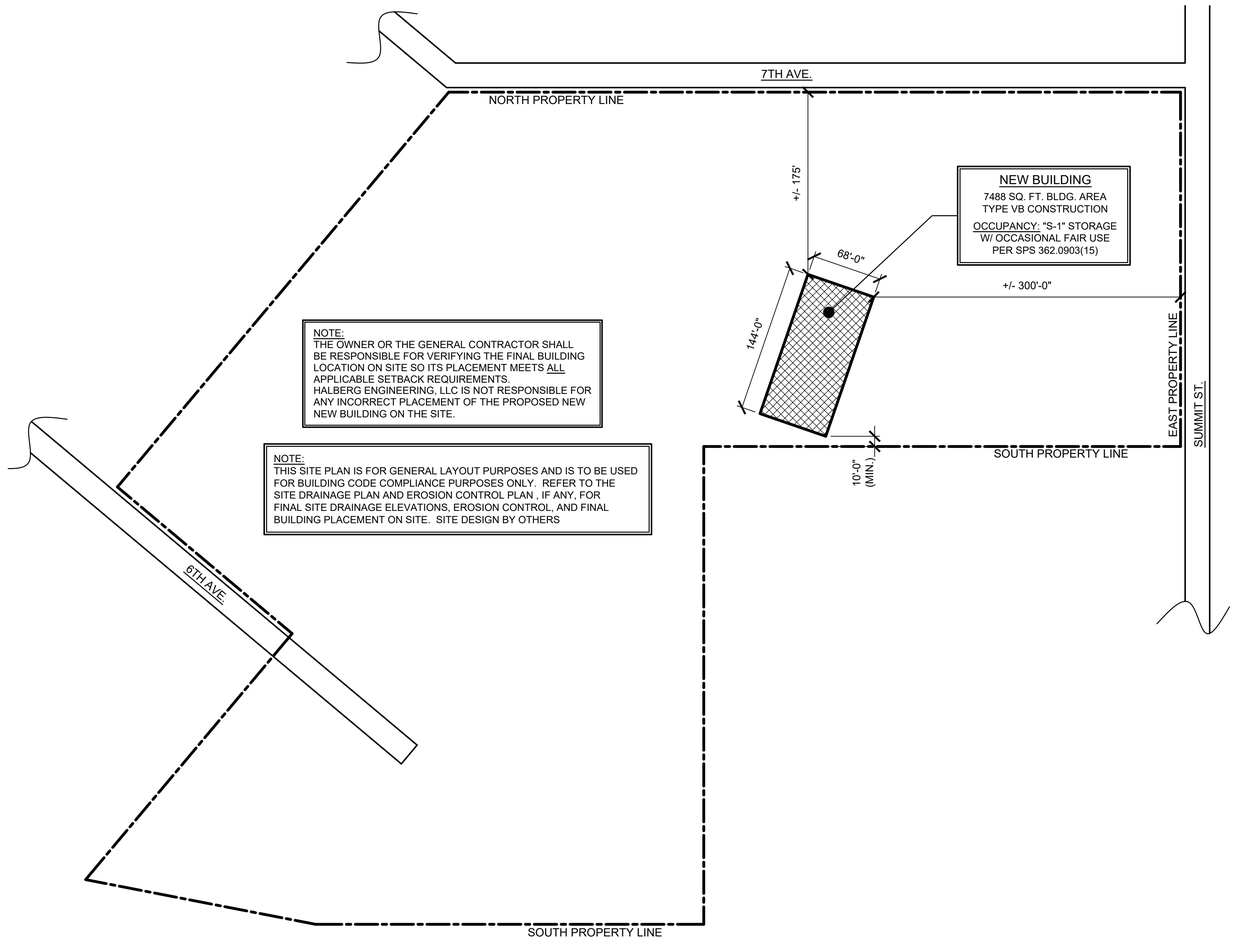
SPS 362.0903 (15)

FAIRGROUND BUILDINGS. Pursuant to s. 101.14 (4), Stats., no city, village, or town may enact or enforce an ordinance that requires a county or organized agricultural society, association, or board to install or maintain an automatic fire suppression system in a building on a fairgrounds if all of the following apply:

(a) The building is open to the public only for seasonal or temporary event use for 180 cumulative days or fewer per year.

(b) Public access to the building is provided by garage style doors that remain open when the building is open to the public.





SITE LAYOUT

SCALE: 1" = 70'-0"

NEW BUILDING FOR:

OSCEOLA FAIRGROUNDS

OSCEOLA, WI

DESIGN PROFESSIONAL:

HALBERG ENGINEERING, LLC
10335N Duffy Road
Hayward WI 54843
P: 866-694-8602
Wisconsin Certificate of Authorization: 3438-11

ENGINEER'S SEAL

SHEET INDEX

GENERAL REQUIREMENTS

NOTES & DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES. ALL MATERIALS AND WORK PERFORMED SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE STATE BUILDING CODE INCLUDING LOCAL ORDINANCES AND AMENDMENTS. ALL MATERIAL SHALL BE FURNISHED AS SHOWN HEREIN UNLESS THE OWNER OR ENGINEER APPROVES EQUAL ALTERNATIVES.
NO CHANGES ARE TO BE MADE TO THESE PLANS WITHOUT THE KNOWLEDGE AND WRITTEN CONSENT OF THE ENGINEER.

THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO BRACING AND SHORING. OBSERVATION VISITS MADE TO THE SITE (IF ANY) BY THE ENGINEER AND/OR THE ENGINEER'S REPRESENTATIVE(S) SHALL NOT INCLUDE INSPECTION OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES.

PROVIDE FIRE EXTINGUISHERS PER IBC906.1 & 906.2. FIRE EXTINGUISHERS SHALL BE SELECTED, INSTALLED, AND MAINTAINED IN ACCORDANCE WITH IFC 906 AND NFPA 10. ANY QUESTIONS ABOUT HOW TO APPLY THESE REQUIREMENTS TO THE SPECIFIC PROJECT SHOULD BE RESOLVED IN CONSULTATION WITH THE FIRE INSPECTOR AND/OR THE PRIMARY RESPONSE FIRE DEPARTMENT FOR THE PROPERTY.

DOORS
THERE SHALL BE A FLOOR OR LANDING PROVIDED ON EA. SIDE OF DOOR. SUCH FLOOR OR LANDING TO BE AT THE SAME ELEV. ON EA. SIDE OF DOOR

GENERAL TRUSS INSTALLATION NOTES:
TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO, AND FOLLOW, THE LATEST EDITION OF BCSI (BUILDING COMPONENT SAFETY INFORMATION, BY "TPI" AND "SBCA") FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. TRUSS INSTALLER OR THE GENERAL CONTRACTOR SHALL PROVIDE TEMPORARY AND PERMANENT BRACING OF THE TRUSS SYSTEM (INCLUDING CHORDS AND WEBS) IN ACCORDANCE WITH BCSI GUIDELINES (UNLESS THESE PLANS SHOW STRICTER REQUIREMENTS, IN WHICH CASE THE MORE STRICTER REQUIREMENTS SHALL BE USED). LOCATIONS SHOWN FOR PERMANENT LATERAL RESTRAINT OF TRUSSES ON THE TRUSS MANUFACTURER'S ENGINEERED TRUSS DESIGN DRAWINGS SHALL HAVE THE APPROPRIATE BRACING INSTALLED PER BCSI (SECTIONS B3, B7, OR B10, AS APPLICABLE)

**A COPY OF THESE APPROVED PLANS AND APPROVAL LETTER SHALL BE ON-SITE DURING CONSTRUCTION AND OPEN TO INSPECTION BY AUTHORIZED REPRESENTATIVES OF THE DEPARTMENT, WHICH MAY INCLUDE LOCAL INSPECTORS. ALL PERMITS REQUIRED BY THE STATE OR LOCAL MUNICIPALITY SHALL BE OBTAINED PRIOR TO COMMENCEMENT OF CONSTRUCTION / INSTALLATION / OPERATION.

NOTE:
ALTHOUGH EVERY EFFORT HAS BEEN MADE IN PREPARING THESE PLANS AND CHECKING THE DETAILS, BOTH THE OWNER AND CONTRACTOR OR SUBCONTRACTOR MUST CHECK ALL DIMENSIONS AND DETAILS FOR ACCURACY. I AM NOT RESPONSIBLE FOR THE CORRECTNESS OF THESE PLANS. THEY ARE COMPLETE CONSTRUCTION TO ALL APPLICABLE STATE, LOCAL, AND OTHER CODE REQUIREMENTS.

HALBERG ENGINEERING LLC

PROJECT: NEW BUILDING

OWNER: OSCEOLA FAIR OSCEOLA, WI

SITE INFORMATION:

NAME: OSCEOLA FAIRGROUNDS
ADDRESS: ...
CITY, STATE, ZIP: OSCEOLA, WI
MUNICIPALITY: VILLAGE OF OSCEOLA, WI
COUNTY: POLK COUNTY

REVISIONS

DATE	DESCRIPTION	BY
10/24/23	VILLAGE APPROVAL DRAWINGS	BVL
8/22/24	PRELIMINARY DRAWINGS	AJH
	ISSUED FOR STATE PLAN APPROVAL	AJH

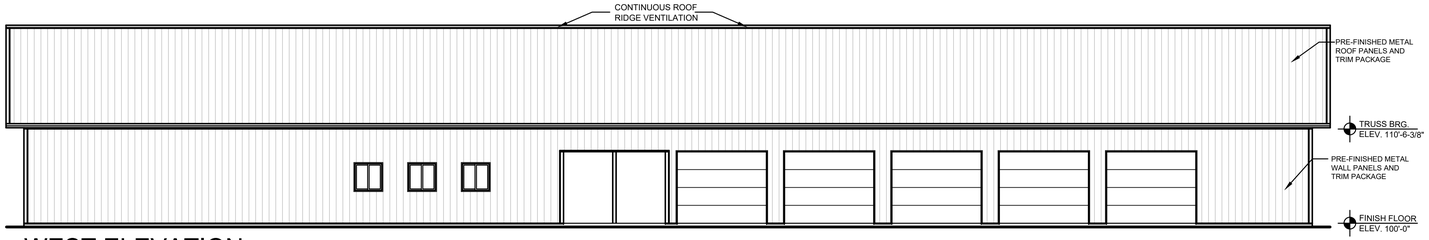
DRAWN BY: BVL

DATE DRAWN: MARCH 2, 2024

PROJECT MANAGER: BRYAN RADDATZ

JOB NUMBER: 24A05

SHEET OF 1 4



WEST ELEVATION
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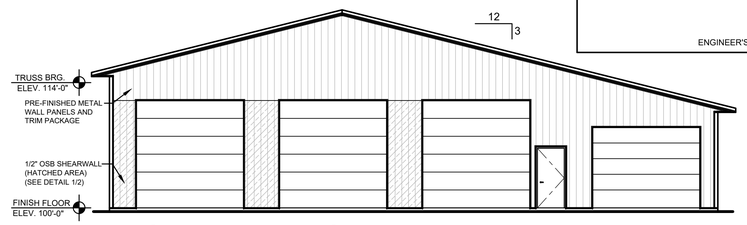

NORTH ELEVATION
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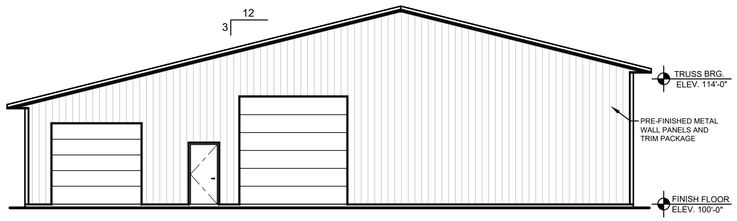
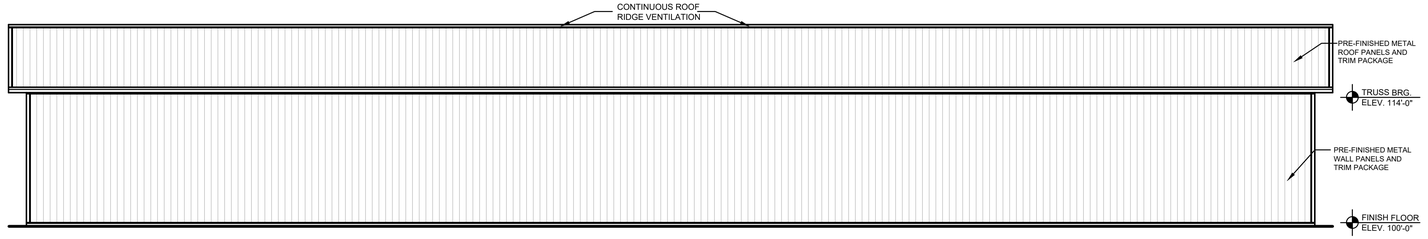
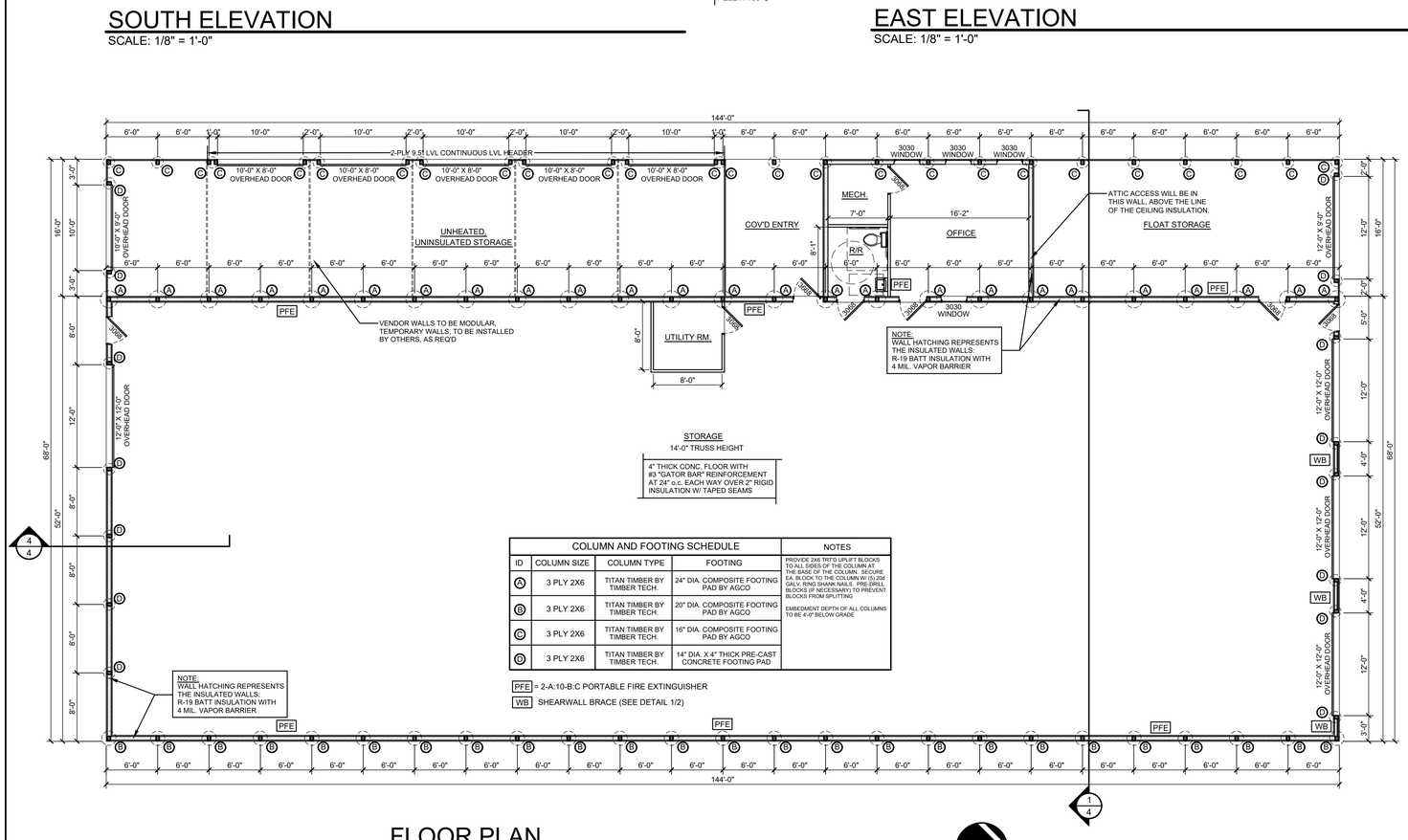


SOUTH ELEVATION
SCALE: 1/8" = 1'-0"



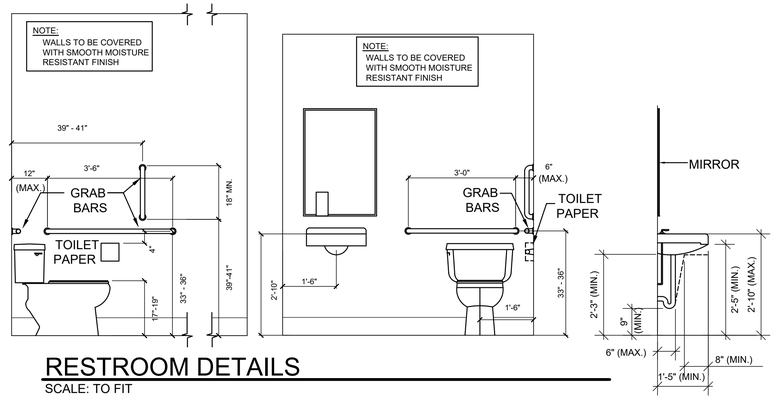
EAST ELEVATION
SCALE: 1/8" = 1'-0"



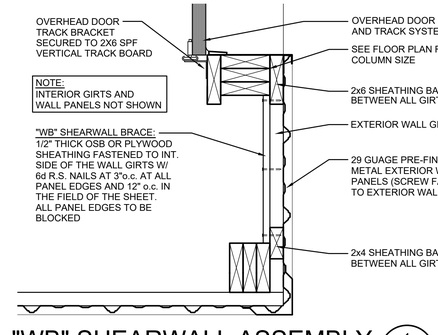
COLUMN AND FOOTING SCHEDULE				NOTES
ID	COLUMN SIZE	COLUMN TYPE	FOOTING	
A	3 PLY 2X6	TITAN TIMBER BY TIMBER TECH.	24" DIA. COMPOSITE FOOTING PAD BY AGCO	PROVIDE 2X6 TRTO UPLIFT BLOCKS TO ALL SIDES OF THE COLUMN AT THE BASE OF THE COLUMN. SECURE EA. BLOCK TO THE COLUMN W/ (5) 20d GALV. RING SHANK NAILS. PRE-DRILL BLOCKS (IF NECESSARY) TO PREVENT BLOCKS FROM SPLITTING EMBEDMENT DEPTH OF ALL COLUMNS TO BE 4'-0" BELOW GRADE
B	3 PLY 2X6	TITAN TIMBER BY TIMBER TECH.	20" DIA. COMPOSITE FOOTING PAD BY AGCO	
C	3 PLY 2X6	TITAN TIMBER BY TIMBER TECH.	16" DIA. COMPOSITE FOOTING PAD BY AGCO	
D	3 PLY 2X6	TITAN TIMBER BY TIMBER TECH.	14" DIA. X 4" THICK PRE-CAST CONCRETE FOOTING PAD	

PFE = 2-A:10-B:C PORTABLE FIRE EXTINGUISHER
WB SHEARWALL BRACE (SEE DETAIL 1/2)

FLOOR PLAN
SCALE: 1/8" = 1'-0"



RESTROOM DETAILS
SCALE: TO FIT



"WB" SHEARWALL ASSEMBLY
SCALE: 1-1/2" = 1'-0"

ENGINEER'S SEAL

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HALBERG ENGINEERING LLC

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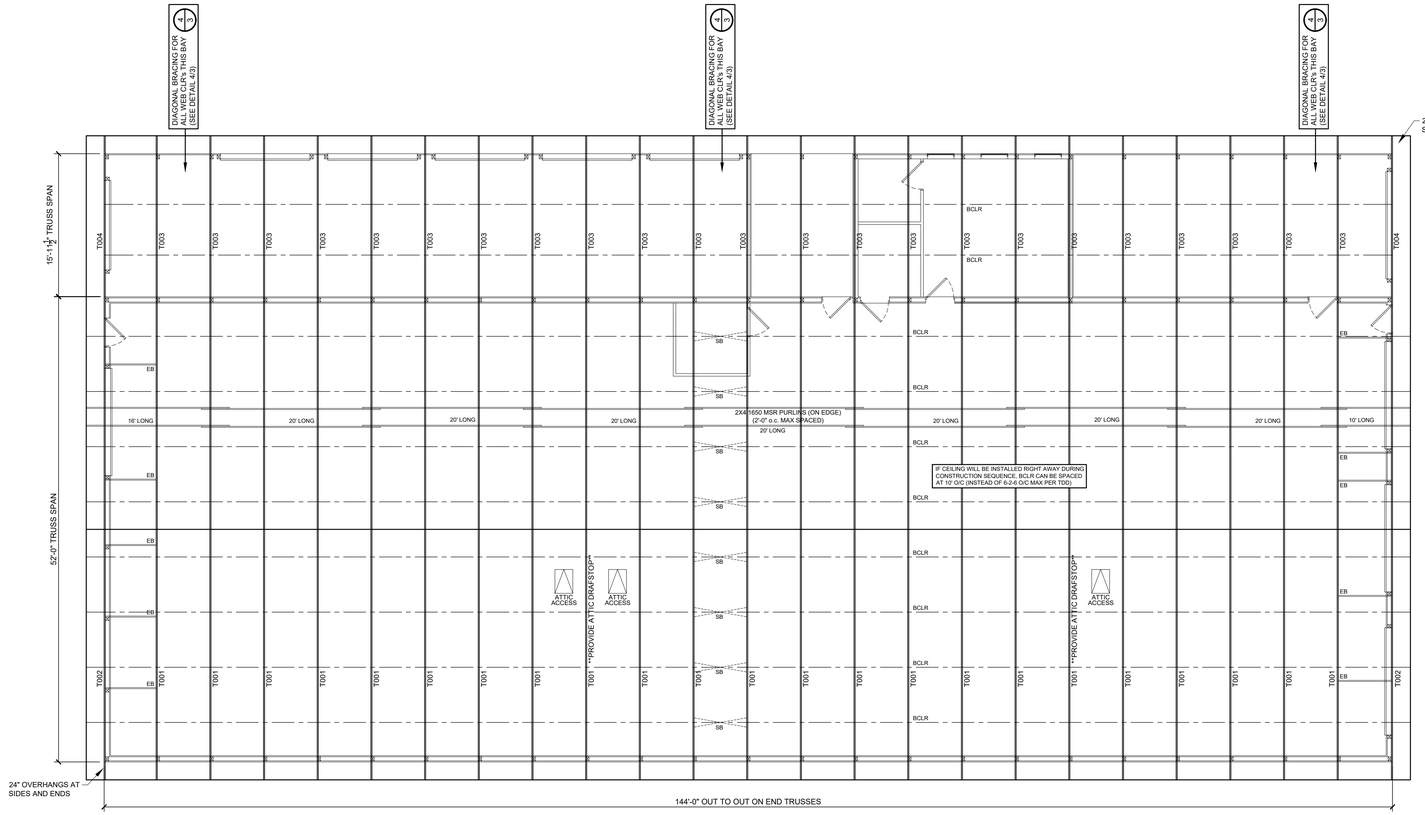
JOB NUMBER: 24A05

SHEET OF 2 4

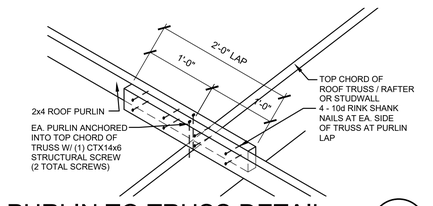


EB: END BRACE (SEE DETAIL 2/3 FOR ASSEMBLY)
SB: SWAY BRACE (SEE DETAIL 3/3 FOR ASSEMBLY)
BCLR: BOTTOM CHORD LATERAL RESTRAINT

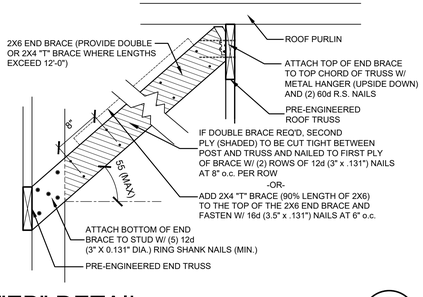
T001: 52'-0" ROOF TRUSS (6'-0" o.c.)
T002: 52'-0" END TRUSS
T003: 15'-11-1/2" MONO SLOPE TRUSS
T004: 15'-11-1/2" MONO SLOPE END TRUSS

ROOF PLAN
SCALE: 1/8" = 1'-0"

N



PURLIN TO TRUSS DETAIL
SCALE: 1" = 1'-0"
1/3



"EB" DETAIL
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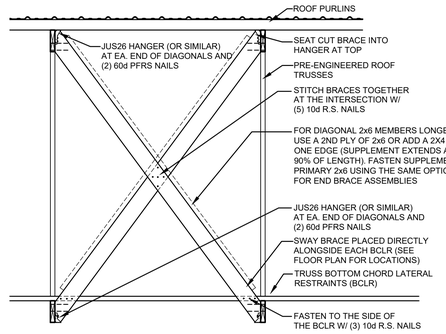
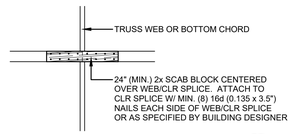
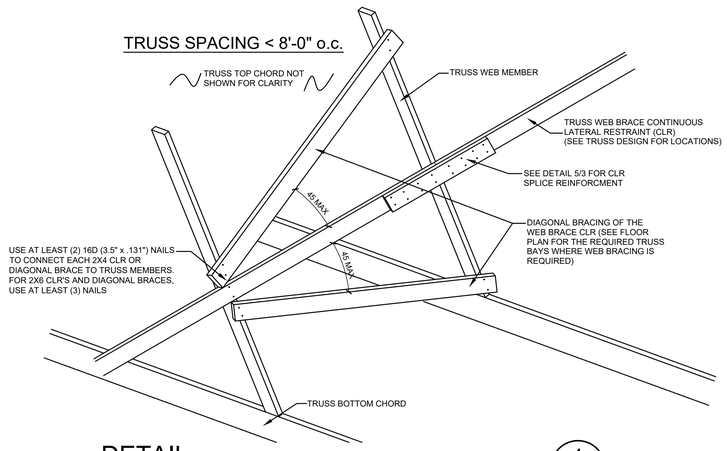
2/3



"SB" DETAIL
SCALE: 1/2" = 1'-0"
3/3



DETAIL
SCALE: 1/2" = 1'-0"
5/3



DETAIL
SCALE: 1/2" = 1'-0"
4/3

ENGINEER'S SEAL

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HALBERG ENGINEERING LLC

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REVISIONS

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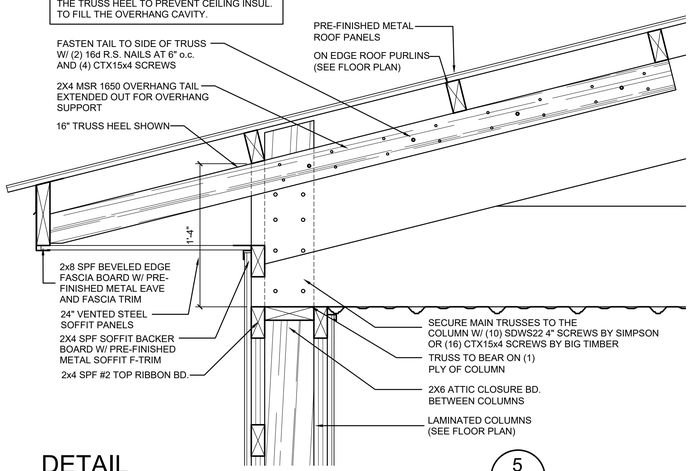
DRAWN BY: BVL
DATE DRAWN: MARCH 2, 2024
PROJECT MANAGER: BRYAN RADDATZ
JOB NUMBER: 24A05

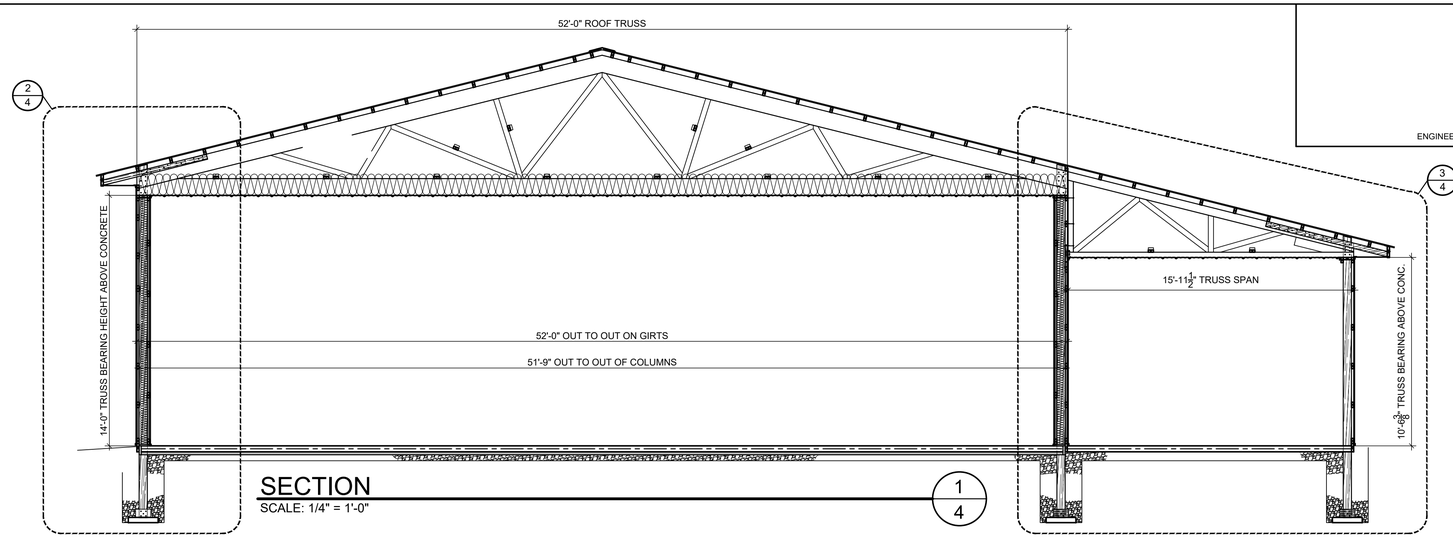
SHEET OF 3 4

NOTE:
WALL AND CEILING INSULATION IS NOT SHOWN FOR CLARITY. PROVIDE INSULATION STOP AT THE TRUSS HEEL TO PREVENT CEILING INSUL. TO FILL THE OVERHANG CAVITY.



DETAIL
SCALE: 1-1/2" = 1'-0"

5/4



SECTION
SCALE: 1/4" = 1'-0"

1/4

2/4

3/4

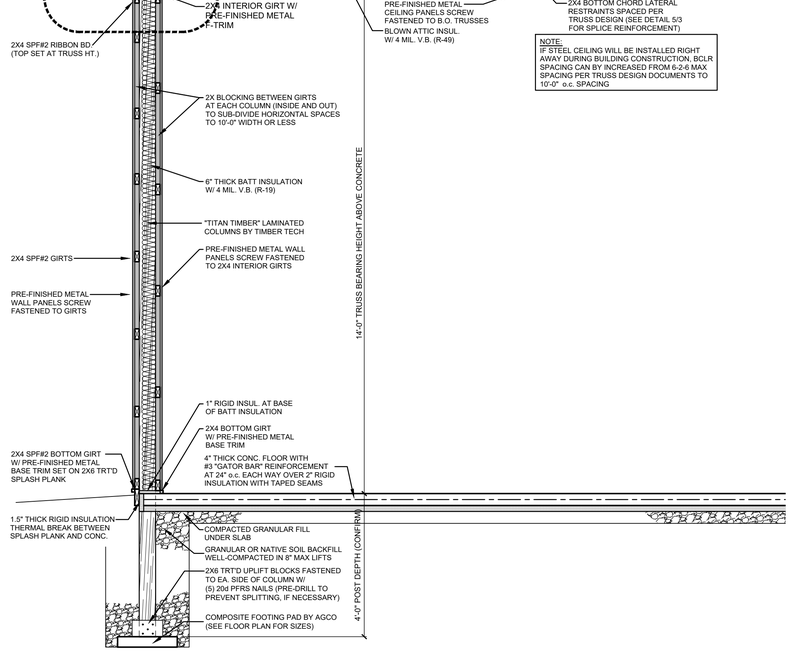
ENGINEER'S SEAL

NOTE:
ALTHOUGH EVERY EFFORT HAS BEEN MADE IN PREPARING THESE PLANS AND CHECKING THEM FOR ACCURACY, THE CONTRACTOR OR SUBCONTRACTOR MUST CHECK ALL DIMENSIONS IN THE FIELD AND BE RESPONSIBLE FOR THE SAME. ALL SUBCONTRACTORS SHALL COMPLETE CONSTRUCTION TO ALL APPLICABLE STATE, LOCAL, AND OTHER CODE REQUIREMENTS.

HALBERG ENGINEERING LLC

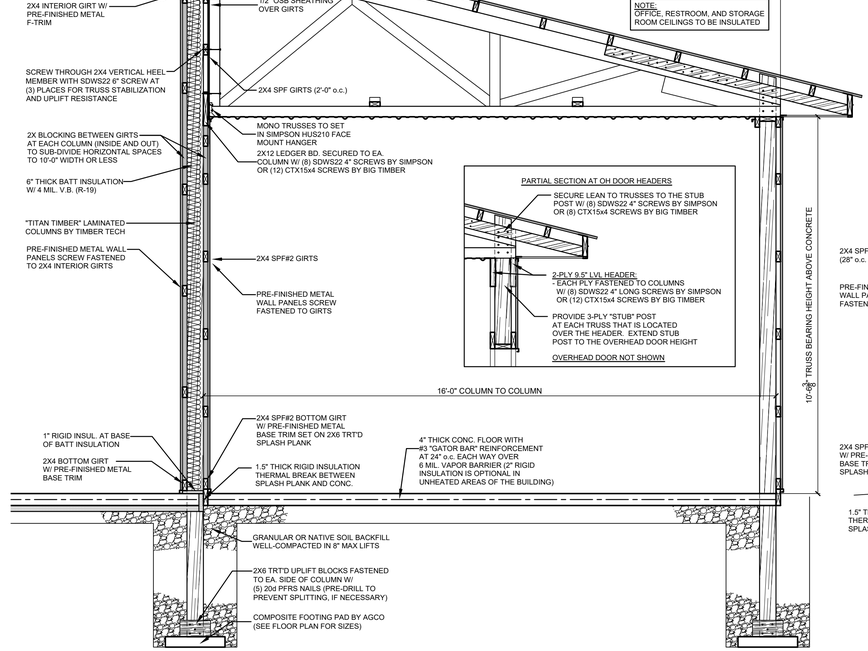
PROJECT: NEW BUILDING

OWNER: OSCEOLA FAIR
OSCEOLA, WI



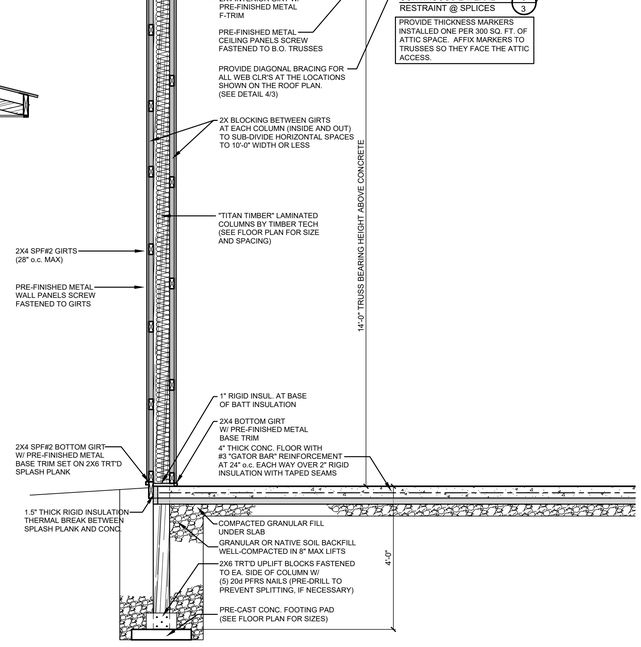
DETAIL
SCALE: 1/2" = 1'-0"

2/4



DETAIL
SCALE: 1/2" = 1'-0"

3/4



SECTION
SCALE: 1/2" = 1'-0"

4/4

SITE INFORMATION:

NAME:	OSCEOLA FAIRGROUNDS
ADDRESS:	...
CITY, STATE, ZIP:	OSCEOLA, WI
MUNICIPALITY:	VILLAGE OF OSCEOLA, WI
COUNTY:	POLK COUNTY

REVISIONS

DATE:	DESCRIPTION	BY
10/24/23	VILLAGE APPROVAL DRAWINGS	BVL
2/2/24	PRELIMINARY DRAWINGS	AJH
	ISSUED FOR STATE PLAN APPROVAL	AJH

DRAWN BY: BVL
DATE DRAWN: MARCH 2, 2024
PROJECT MANAGER: BRYAN RADDATZ
JOB NUMBER: 24A05

SHEET OF 4